Microstructural Origins of Cement Paste Degradation by External Sulfate Attack

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Abstract

A microstructure model has been applied to simulate near-surface degradation of portland cement paste in contact with a sodium sulfate solution. This new model uses thermodynamic equilibrium calculations to guide both compositional and microstructure changes. It predicts localized deformation and the onset of damage by coupling the confined growth of new solids with linear thermoelastic finite element calculations of stress and strain fields. Constrained ettringite growth happens primarily at the expense of calcium monosulfoaluminate, carboaluminate and aluminum-rich hydrotalcite, if any, respectively. Expansion and damage can be mitigated chemically by increasing carbonate and magnesium concentrations or microstructurally by inducing a finer dispersion of monosulfate.

Key words: cement paste, sulfate attack, microstructure, computer modeling, expansion, degradation

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1 1. Introduction

The degradation of cementitious materials by external sulfate attack is 2 one of the most frequently cited causes of service life reduction of concrete 3 structures. The importance of sulfate attack is attested by the multitude 4 of experimental and theoretical studies on its origin and manifestations that 5 have been published just in the last decade [1-22]. Such investigations have 6 consistently indicated that sulfate attack is a complicated, multiscale and 7 multiphysics process involving the coupling of physical, chemical, and me-8 chanical interactions. 9

The formation of ettringite [23-25] and sometimes gypsum [26] are usu-10 ally thought to be responsible for significant volumetric expansion and struc-11 tural damage during sulfate attack. Several of the proposed mechanisms 12 of ettringite-related expansion (e.q., solid state conversion [27, 28], volume13 increase [29], colloidal swelling [30]) often do not match experimental obser-14 vations and measurements [15, 21]. More recently, crystallization pressure 15 theory has shown that stress exerted on pore walls by impingement of grow-16 ing crystals is significant only in confined spaces and when the surrounding 17 solution is highly supersaturated with respect to those crystals [31–33]. Ac-18 cordingly, macroscopic stress during sulfate attack is determined more by 19 the locations of active ettringite growth in the microstructure rather than 20 the total volume of ettringite formed [18, 21]. That is, the origin and ex-21 tent of sulfate-induced expansion is the transformation to ettringite of AFm 22 phases that are dispersed within the fine porosity of C–S–H gel [15]. 23

Various approaches have been used to simulate the effects of sulfate attack. A multiionic transport model [34] has been used to simulate the evo-

lution of mineral phases in cement-based materials exposed to aggressive 26 environments such as sulfate solution [35, 36]. Infilling of capillary porosity 27 by precipitated solids and the resulting displacement of liquid from the ma-28 terial has been simulated by a finite-difference approach for nonsteady-state 29 diffusion and reaction of ions in cementitious composites [10, 37]. Other 30 reaction-diffusion models have been applied to the corrosion of building ma-31 terials subjected to sulfate attack [38], although models of this kind require 32 knowledge of the transport properties and porosity within the material. Ex-33 pansion and damage have been simulated using continuum damage mechanics 34 at the millimeter length scale [4, 5, 9, 16, 17]. In particular, the finite element 35 model developed by Idiart [17] simulates the millimeter-scale distribution of 36 ettringite and development of expansion cracks around aggregates embedded 37 within an homogenized cementitious binder. 38

Modeling of sulfate attack at the scale of the cementitious binder mi-30 crostructure has not been reported, but nevertheless could be valuable for 40 deepening our understanding of the origin and mechanisms of sulfate-induced 41 degradation. In this paper, we report a first step in this direction. A 42 thermodynamic-microstructural model of hydration [39] is coupled to a linear 43 thermoelastic finite element model [40] to simulate microstructural evolution 44 by external sulfate attack and to track the microscopic stress field that de-45 velops by constrained ettringite growth in the binder porosity. The model 46 is applied here to simulate near-surface sulfate attack of Type I ordinary 47 portland cement binder in a sodium sulfate solution. We also investigate the 48 sensitivity of the model predictions to the spatial distribution of aluminate-49 bearing phases in the microstructure. Finally, the model is used to simulate 50

and compare the differences in the progress of sulfate attack among three
types of cement having different aluminate contents.

⁵³ 2. Model Description and Technique

54 2.1. Simulating hydration

The model used here is an extension of the Thermodynamic Hydration 55 and Microstructure Evolution (THAMES) model recently developed at the 56 National Institute of Standards and Technology (NIST) [39]. THAMES has 57 three main components: (1) a kinetic model of cement clinker phase disso-58 lution, which determines the time dependence of elemental concentrations 59 in the pore solution [41]; (2) thermodynamic speciation calculations to de-60 termine the equilibrium composition and abundance of phases other than 61 clinker minerals, including the pore solution [42-44]; and (3) a 3D digital 62 image model for spatially distributing these various phases to represent the 63 microstructure of the binder. Full details about how these three components 64 work together is provided elsewhere [39, 45]. Briefly, each a 1 μ m³ of space 65 is represented by a voxel in the microstructure having an integer identifier 66 unique to the phase occupying that volume. Each voxel sharing one or more 67 faces with a different phase is identified as an "interface" voxel and stored in 68 lists of sites eligible for dissolution or growth of that phase. The list members 60 are evaluated for their potential to dissolve or grow, respectively, by assigning 70 an "affinity" at each interface voxel that characterizes the relative tendency 71 of a given phase to grow in the environment at that site. A negative affinity 72 between two phases means that increases in the interface between them is 73 unfavorable, while a positive affinity between to phases tends to promote 74

the increase of that interface. This concept of affinity does not have any 75 well-defined thermodynamic significance in terms of crystal growth theory, 76 but is used merely as a convenience to generate different growth morpholo-77 gies. In the future, it may be possible to establish a stronger link between 78 the affinities and physical parameters such as interfacial energy and growth 79 mobility of interfaces as a function of orientation. Consistent with the ob-80 servations of Schmidt and coworkers [15], we assume that the transformation 81 to ettringite of aluminate-bearing phases that are finely dispersed within C-82 S-H give rise to stress fields that are responsible for expansion. Therefore, 83 we adopt affinity values that promote the fine intergrowth with C–S–H of 84 calcium monosulfoaluminate (hereafter called monosulfate), calcium mono-85 and hemicarboaluminate (hereafter called monocarbonate and hemicarbon-86 ate, respectively), and hydrotalcite. 87

⁸⁸ 2.2. Simulating interaction with Na_2SO_4

Hydration was stopped at 100 d of hydration and the simulation of sul-89 fate attack was started. 100 d of hydration was chosen arbitrarily, but we 90 note that the degree of hydration at this time is 0.90, based on mass of 91 clinker consumed, so further hydration happens quite slowly. Therefore, ig-92 noring further hydration from that point is likely a good approximation. The 93 simulation begins by flushing the capillary pore volume with a $0.1 \,\mathrm{mol}\,\mathrm{kg}^{-1}$ 94 Na_2SO_4 solution, using an algorithm reported recently [45]. The total mass 95 of water in the aqueous solution phase is first retrieved from the current 96 equilibrium state, together with the molal concentrations of the primary so-97 lute species $[SO_4^{2-}]$ and $[Na^+]$. From these data, calculations are made of 98 the number of moles of each of these ions that must be added to, or re-90

¹⁰⁰ moved from, the solution to increase or decrease, respectively, their molal ¹⁰¹ concentration to prescribed initial values prior to each cycle's equilibration ¹⁰² step. $[K^+]$ and $[Ca^{2+}]$ were also set in the same manner to 0.5 mmol kg⁻¹ and ¹⁰³ 0.1 µmol kg⁻¹, respectively, prior to equilibration, a minimum value chosen ¹⁰⁴ to avoid numerical instabilities in the thermodynamic calculations while still ¹⁰⁵ enabling the process of calcium leaching which is observed in sulfate attack ¹⁰⁶ experiments [47, 48].

The thermodynamic system is re-equilibrated with the newly flushed sul-107 fate solution. Phase transformations are assumed to be dictated by ther-108 modynamic equilibrium between the solid phases and solution, altering both 109 the solution speciation and the volumes of different solid phases. The mi-110 crostructure is updated according to the aforementioned rules, adding or 111 subtracting voxels of each phase corresponding to that phase's net volume 112 fraction change. The model repeats the process of flushing, re-equilibration, 113 and microstructure updates. The physical time associated with each of these 114 flushing cycles would be determined by (1) the time required to exchange 115 the pore solution with fresh solution, and (2) the time required for the fresh 116 solution to reach equilibrium with the remaining solids. Either of these pro-117 cesses could, in principle, control the time scale of a given flushing cycle. 118 In typical experiments on sulfate attack, the specimen is submerged in a 119 sulfate solution of fixed composition, so solute diffusion from the interior 120 capillary pore space probably controls the rate of degradation everywhere 121 except very near the exterior surface, where the rate is likely controlled by 122 mineral dissolution or growth. The present study does not attempt to cap-123 ture the complexities of diffusion-controlled solute migration to the interior 124

of the specimen. Instead, our purpose is to explore the overall course of early 125 degradation at the specimen surface (*i.e.*, the first $100 \,\mu\text{m}$ layer) in terms 126 of microstructure development, expansion, and the onset of damage. With 127 this microstructure modeling capability established, a subsequent study will 128 explore microstructure development and expansion as a function of depth 129 by coupling the thermodynamic-microstructure model to a reaction-diffusion 130 framework similar to that employed by others for modeling at the macro-131 scopic scale [17]. 132

133 2.3. Elastic strain and stress calculations

Ettringite has a greater molar volume than the aluminate-bearing phases from which it forms, and we assume it to be the only product that causes expansion during sulfate attack. This assumption agrees with previous studies even though no direct relation has been established between the abundance of ettringite and expansion of the sample.

Crystallization pressure theory [32] predicts that stress is exerted on pore 139 walls by crystals formed within confined solutions that are supersaturated 140 with respect to continued growth in bulk solution. Crystallization is therefore 141 thermodynamically favored to happen in larger pores where this pressure is 142 reduced. However, the diffusion of ions can limit the rate at which crystalliza-143 tion happens in larger pores, and significant transient pressure can therefore 144 be generated within fine pores during ettringite growth [33]. Considering the 145 low mobility of aluminate ions, ettringite is assumed in this model to form at 146 the places where aluminum sources dissolve during sulfate attack. However, 147 we are unable to directly model crystallization pressure because (1) the equi-148 librium bulk thermodynamic assumption of our approach guarantees that 149

the solution is never supersaturated with respect to any solid phase in the microstructure, and (2) the 1 μm spatial resolution of our microstructure model is too coarse to explicitly include the finest pore dimensions within the hydration product where crystallization pressures would be the greatest.

Without explicitly modeling crystallization pressure distribution through-154 out the microstructure, we approximate the microstress development as re-155 sulting from local misfit strains that occur when the stress-free volume change 156 of ettringite is greater than the local volume available for it. For example, 157 consider the local microstructure configuration illustrated in Fig. 1. One 158 voxel of monosulfate is embedded within C–S–H hydration product with 159 only one neighbor of capillary porosity. The reaction of this monosulfate 160 voxel with sulfate solution precipitates ettringite at the same site. The mo-161 lar volume of ettringite is 1.28 times larger than the molar volume of mono-162 sulfate [49], so the reaction of one monosulfate voxel should correspond to 163 the growth of 2.28 voxels of stress-free ettringite. Therefore, if at least two 164 voxels of capillary porosity are adjacent to the reacted monosulfate voxel, we 165 (1) replace the monosulfate voxel with one of ettringite, (2) replace one of the 166 adjacent capillary pore voxels with ettringite, and (3) replace one more adja-167 cent capillary pore voxel with ettringite with a probability of 0.28. However, 168 if fewer than two capillary pore voxels are adjacent to the reacted monosul-169 fate voxel, as depicted in the figure, then the ettringite cannot grow in a 170 stress-free state; instead, a local volumetric strain of $\epsilon = -0.28$ or $\epsilon = -1.28$ 171 is assigned to the newly formed ettringite depending on whether there are 172 one or zero adjacent pore voxels, respectively. The negative values mean 173 that ettringite is under a compressive strain. Misfitting ettringite thus expe-174

riences a compressive (negative) stress exerted on it by its surroundings, and
simultaneously exerts a tensile (positive) stress on those surroundings. That
is, a heterogeneous stress field exists throughout the microstructure during
any given time step.

This stress field is calculated after the misfit strains have been assigned by 180 coupling the microstructure to a 3D linear thermoelastic finite element (FE) 181 model. The FE calculation is optimized for computing elastic properties of 182 random composite media [40], and uses the full 3D microstructure as input 183 by representing each voxel as a trilinear cubic mesh element. Each element 184 is assigned values for its average isotropic bulk and shear moduli based on 185 Voigt-Reuss-Hill bounds for the phase in that element; values for most cement 186 phases have been generally accepted in the literature and are tabulated in 187 Ref. [50]. 188

The elastic energy, U, of the microstructure with a given microstrain distribution is

$$U = \frac{1}{2} \int_{V} dV \left[(\boldsymbol{\epsilon} - \mathbf{e}) : \mathbf{C} : (\boldsymbol{\epsilon} - \mathbf{e}) \right]$$
(1)

where $\boldsymbol{\epsilon}$ is the local strain tensor, \mathbf{C} is the stiffness tensor, and \mathbf{e} is the local misfit strain tensor. The condition of elastic equilibrium is that U be minimized with respect to the displacement field, which is met if $\partial U/\partial \mathbf{u} = 0$ for the displacement vector \mathbf{u} evaluated at each node of the finite element mesh. The finite element calculation uses a conjugate gradient method to determine the energy-minimizing displacement field that satisfies the boundary conditions, as detailed in Ref. [40]. The equilibrium displacement field determines the equilibrium strain field according to $\boldsymbol{\epsilon} \equiv \partial \mathbf{u}/\partial \mathbf{x}$, where \mathbf{x} is the position vector. In turn, the equilibrium strain at a point determines the stress tensor at that point according to $\boldsymbol{\sigma} \equiv \mathbf{C} : (\boldsymbol{\epsilon} - \mathbf{e})$. This FE implementation model has been verified by comparison to theoretical results of composite theory [51] and used extensively for random composite media in general [52–54] and for portland cement paste microstructures in particular [50, 55, 56].

The average stress in each voxel is calculated by diagonalizing the stress 204 tensor averaged over the eight FE nodes at the voxel corners. Damage is 205 assumed to occur locally within the microstructure at any voxel where the 206 tensile (positive) stress exceeds a critical value $\sigma_c(p)$ that depends on the 207 phase p at the voxel. Clinker phases are assumed to be the most resistant to 208 tensile failure by assigning each a critical tensile strength of $\sigma_c = 10$ MPa. 209 Hydration product phases are each assigned a critical tensile strength based 210 on their elastic moduli. For simplicity, C–S–H, ettringite and hydrotalcite 211 are each tentatively assigned $\sigma_c = 1$ MPa, considering their relatively lower 212 Young's modulus. All other hydrates are assigned $\sigma_c = 5$ MPa, which is 213 close to the typical measured tensile strength of ordinary portland cement 214 paste [57, 58]. If the tensile stress for any voxel exceeds σ_c , that voxel is 215 designated as "damaged". This approach provides an indication of regions 216 in the microstructure that are most susceptible to loss of mechanical integrity 217 due to sulfate attack, but it does not capture the development and coalescence 218 of microcrack networks into larger cracks. Any such modeling of fracture 219 would require either a finer and adaptive mesh that could conform to crack 220 walls as they propagate, or an extended finite element method on a regular 221 mesh [59]. 222

Note that this method of coupling FE calculations to microstructure development automatically accounts for the changing effective elastic stiffness of the binder, which influences the amount of expansion obtained for a given induced stress. As a result, this approach has the advantage of simultaneously predicting not only the influence of binder chemistry on the phase evolution during external sulfate attack, but also the influence of chemistry and microstructure on the evolving resistance of the binder to expansion.

As a summary of the model description given in these sections, Fig. 2 shows an overview of the general algorithm.

[Figure 2 about here.]

233 3. Results and discussion

232

234 3.1. Evolution of pore solution and solids

A Type I ordinary portland cement (proficiency sample cement 168 from 235 the Cement and Concrete Reference Laboratory and hereafter designated as 236 CCRL 168) is used first. The cement has a fineness of 408 $m^2/kg \pm 10 m^2/kg$ 237 as measured by ASTM C 204 [60], with a median particle size of 13.7 μ m as 238 measured by laser scattering from a dilute powder suspension in isopropanol 239 following AASHTO T 353-14 [61]. The mineralogical composition of the 240 cement is shown in Table 1, and is based on average mass fractions obtained 241 using X-ray powder diffraction with Rietveld refinement [39]. 242

²⁴³ A 3D digital-image representation of the cement paste microstructure, ²⁴⁴ based on CCRL 168 and with w/c = 0.45, was constructed using the Virtual ²⁴⁵ Cement and Concrete Testing Laboratory (VCCTL) software [62]. Saturated hydration of each paste for 100 d at a fixed temperature of 25 °C was simulated using the methods described in the previous section. The hydrated
microstructure was then subjected to the iterative sulfate attack algorithm.
The microstructure, solid phase composition, and solution composition were
tracked continuously during the simulation.

²⁵¹ [Table 1 about here.]

Before analyzing the predicted evolution of solid phases in the microstruc-252 ture, note that a number of those phases are not stoichiometric but rather 253 are solid solutions with variable composition. For example, the crystal struc-254 ture of ettringite can have a fraction of its Al^{3+} sites substituted by Fe^{3+} 255 because of their identical charge and similar ionic radii. To account for this, 256 the thermodynamic database models the ettringite phase as a solid solution 257 of two stoichimetric end members, one with 0 % Fe substitution and the 258 other with 100 % Fe substitution. This general interchangeability of Al and 259 Fe is also found in monosulfate, the carboaluminate phases, and in hydro-260 talcite. Table 2 shows the end member compositions and molar volumes of 261 the aluminum- and iron-bearing hydrates in the thermodynamic database. 262 The partial substitution of iron in these phases can change their relative 263 thermodynamic stabilities in different environments. Therefore, although we 264 do not explicitly plot the time-dependent chemical compositions of these hy-265 drates, we note occasions where dissolved iron has a significant influence on 266 the course of sulfate attack. 267

Fig. 3 shows the predicted mass change of solid hydrate phases with progressive sulfate attack of the paste. Ettringite increases significantly from the first introduction of sodium sulfate solution. Closer inspection of Fig. 3(a)
indicates that, from the perspective of phase stability, sulfate attack can be
roughly divided into four stages:

- Stage 1: Growth of ettringite at the expense of monosulfate;
- Stage 2: Growth of ettringite at the expense of carboaluminate phases;
- Stage 3: Growth of ettringite at the expense of a portion of hydrotalcite; and
- Stage 4: Growth of gypsum at the expense of portlandite.
- [Figure 3 about here.]
 - [Table 2 about here.]
- 280 First stage: transformation of monosulfate

279

Monosulfate dissolves continuously in this stage, being replaced by a greater volume of ettringite. The first stage ends when monosulfate has dissolved completely from the system.

The pore solution at the end of hydration has a lower $[Na^+]$ than the 284 external sulfate solution, so exposure to the sulfate solution causes a slight 285 increase in $[Na^+]$ in the pore solution, as shown in Fig. 4(a). Conversely, 286 both K^+ and OH^- are extracted from the pore solution during each solution 287 flushing cycle (Fig. 4(b)), so the pore solution pH decreases mildly with 288 time, especially during the first and second stage, as shown in Fig. 5. Final 289 dissolution of monosulfate is attended by a rapid increase in both ettringite 290 and hemicarbonate, which causes a minor perturbation in both $[Na^+]$ and 291

[K⁺] (Fig. 4). These perturbations occur because the sudden increase in the growth rate of ettringite, each mole of which requires 26 moles of water, significantly reduces the liquid water content in the system.

²⁹⁵ [Figure 4 about here.]

296

[Figure 5 about here.]

²⁹⁷ Second stage: transformation of carboaluminates

The second stage begins after monosulfate has been consumed. Transformation of hemicarbonate, partly to ettringite and partly to monocarbonate, characterizes the second stage shown in Fig. 3(a). Monocarbonate is destabilized and begins dissolving when hemicarbonate is consumed, enabling ettringite growth to continue. The second stage ends with the complete dissolution of monocarbonate, at which point small jumps in $[Ca^{2+}]$ and $[SO_4^{2-}]$ are observed, as shown in Fig. 4(c) and (d).

The relative importance and duration of the second stage depends on the concentration of carbonates in the system, and therefore is expected to increase in importance with service life in environments favoring carbonation. Conversely, binders with little or no limestone content that have been placed in low-CO₂ environments may not exhibit this stage at all.

310 Third stage: transformation of hydrotalcite

Hydrotalcite gradually dissolves in the first two stages, but in the third stage is the only remaining solid that can contribute aluminates to the solution. Therefore, its dissolution is accelerated in the third stage as ettringite continues to grow. In this stage, ettringite grows at a slightly slower rate, leading to the increase of [SO₄²⁻] until all the aluminum-bearing hydrotalcite is consumed, after which ettringite growth ceases. Note that some ironrich hydrotalcite remains in the system because the iron-rich hydrotalcite end member is thermodynamically stable relative to ettringite. Therefore, in CCRL 168, which has a high enough magnesium concentration to form significant quantities of hydrotalcite, the fate of iron is divided between hydrotalcite and ettringite even after prolonged exposure to sulfate solution.

The relative importance of this third stage depends on the concentrations of magnesium available to form hydrotalcite. Systems with dolomitic limestone filler or periclase may therefore exhibit a more prominent third stage than those in which magnesium sources are absent.

326 Fourth stage: leaching of portlandite

The fourth stage begins when all aluminate and iron sources already 327 described have been transformed to ettringite. Without an aluminate source, 328 ettringite stops growing even as more sulfate enters the system. A sharp 329 increase occurs of both $[Ca^{2+}]$ and $[SO_4^{2-}]$. Increasing $[SO_4^{2-}]$ at this stage 330 is consistent with published experiments [21] and implies that ettringite is 331 able to buffer sulfates only as long as aluminates are available to enable its 332 growth. The increase of $[Ca^{2+}]$, on the other hand, is caused by ongoing 333 dissolution of portlandite (see Fig. 3(b)) as the solution pH continues to 334 decrease by influx of sulfate solution. Both $[SO_4^{2-}]$ and $[Ca^{2+}]$ increase until 335 the solution becomes saturated with respect to gypsum, and thereafter the 336 growth of gypsum shown in Fig. 3(b) buffers the solution with respect to 337 both ions and their concentrations remain essentially constant until the end 338 of the simulation (Fig. 4)(c) and (d)). 339

340 3.2. Gypsum formation

Gypsum is often observed to form on the surface of samples exposed to 341 sodium sulfate solutions, and to progress inward with time as severe crack-342 ing and spalling create paths for rapid influx of sulfate solution to the in-343 terior [15, 63]. Formation of gypsum on sample surfaces is due to rapid 344 leaching of near-surface portlandite in the low-pH environment of the sulfate 345 bath, which provides calcium to react with free sulfate in solution. Even 346 so, the current near-surface modeling study predicts the formation of gyp-347 sum only much later in the simulations, and only after ettringite growth has 348 stopped because all aluminate sources have been consumed (see Fig. 3). One 349 reason for this may be that the simulations do not consider the free solution 350 adjacent to the solid surface, where growth conditions for gypsum may be 351 especially favorable relative to ettringite. Within the solid microstructure, 352 however, ettringite is more thermodynamically favored than gypsum while 353 both aluminates and sulfates are available in the pore solution. In other 354 words, the free energy is lower when all the incoming sulfate can incorpo-355 rated to grow more ettringite, rather than being divided between ettringite 356 and gypsum. Another way of viewing this is that the existing AFm and AFt 357 phases in the system buffer both calcium and sulfate at quite low concen-358 trations during the first 100 flushing cycles (see Fig. 4), so that the activity 359 product for gypsum is held well below its solubility product. However, we 360 emphasize that this is a thermodynamic prediction. Gypsum could form 361 near the surface, either in the adjacent solution or on the surface itself, if its 362 growth mechanism was fast enough to rapidly consume both leached calcium 363 and incoming sulfates before they could be incorporated within ettringite. 364

Such a scenario seems plausible because the crystal structure of gypsum is considerably simpler than that of ettringite, and fewer dissolved components must be coordinated to form a unit cell of gypsum than one of ettringite.

368 3.3. Microstructure development

Fig. 6 shows the microstructure of the unhydrated paste, the paste hydrated to a degree of hydration of 0.90, and the phase assemblage and localized deformation at the end of Stage 1 and Stage 2. Areas of likely damage initiation, caused by the local stress exceeding the tensile strength, are shown at the end of Stages 1 and 2 as black voxels in Fig. 7.

- ³⁷⁴ [Figure 6 about here.]
 - [Figure 7 about here.]

375

If expansion is caused by confined growth of ettringite in the available 376 porosity adjacent to dissolving AFm phases, at least two microstructural 377 properties should be important in determining the initiation of damage: 378 (1) the volume of porosity adjacent to aluminate-bearing solid domains into 379 which excess ettringite can grow without constraint; and (2) the dispersion 380 of these phases throughout the microstructure at the beginning of sulfate at-381 tack, which in turn controls the extent of dispersion of transformed ettringite 382 throughout the microstructure. With regard to the latter, we hypothesize 383 that a higher surface-to-volume ratio of monosulfate, in the form of smaller 384 phase domains, leads to a greater fraction of the microstructure's capillary 385 porosity being in close proximity to a monosulfate surface. The availability 386 of nearby capillary porosity can provide a "safety valve" that can accept the 387

excess ettringite volume without increasing the local stress. Therefore, a finer distribution of monosulfate may lead to a smaller degree of expansion and damage for a given amount of monosulfate conversion.

This hypothesis can be tested by tracking the radial distribution of capillary porosity near monosulfate surfaces for different dispersions of monosulfate. We define the radial distribution function of porosity relative to monosulfate, g(r), as the volume of capillary pores at a distance r from any monosulfate surface, $V_p(r)$, normalized by the total capillary pore volume, $V_{p,tot}$, so that

$$g(r) \equiv \frac{V_p(r)}{V_{p,\text{tot}}} \tag{2}$$

Applied to digitized microstructure images, q(r) characterizes the probabil-397 ity of finding a capillary pore voxel at a distance r from a monosulfate voxel, 398 relative to the probability of finding a pore voxel at a random location in 399 the microstructure. That is, for example, g(r) > 1 indicates that pore vox-400 els are relatively more plentiful at a distance r from a monosulfate surface 401 than they are throughout the whole microstructure. We quantify the rela-402 tive availability of local capillary porosity near monosulfate surfaces by first 403 computing q(r) on a given microstructure and then finding its average value 404 for $r \leq 2 \ \mu m$, 405

$$\langle g \rangle \equiv \frac{1}{2} \int_0^2 g(r) \,\mathrm{d}r$$
 (3)

For a digital image microstructure, g(r) can be computed only at discrete distances, that is, the center-to-center distance of voxels that are nearest neighbors $(r = 1 \ \mu\text{m})$, second-nearest neighbors $(r = \sqrt{2} \ \mu\text{m})$, and so on. Therefore, the integral reduces to a sum over the discrete distances $\{1, \sqrt{2}, \sqrt{3}, 2\}$.

Three unhyhdrated microstructures of a CCRL 168 paste were created 410 with w/c = 0.45, each having dimensions 100 μ m on a side. The absolute 411 variation in volume fraction of any phase among the three microstructures 412 was < 0.001. For each unhydrated microstructure, a series of ten differ-413 ent hydration simulations were conducted in which the affinity parameters 414 for both monosulfate and C–S–H were systematically varied to produce hy-415 drated microstructures that differed *only* in the coarseness of the monosulfate 416 dispersion throughout the hydration product. This procedure resulted in a 417 collection of 30 different microstructures in which the volume fractions of solid 418 phases were statistically identical but contained monosulfate domain num-419 ber densities (*i.e.*, number of domains per unit volume) that varied between 420 $1.5 \times 10^7 \,\mathrm{mm^{-3}}$ and $4.5 \times 10^7 \,\mathrm{mm^{-3}}$. Higher number densities correspond to 421 smaller average domain volumes because the total volume of monosulfate is 422 the same in each microstructure at the end of hydration. 423

Each of the 30 hydrated microstructures was then subjected to a series of 424 sulfate attack simulations to determine the influence of monosulfate distri-425 bution on the evolution of strain and damage. Fig. 8 plots $\langle g \rangle$ as a function 426 of the number of monosulfate domains per unit volume at the beginning of 427 Stage 1 sulfate attack. Different symbols are used to represent each of the 428 three beginning microstructures, so that the influence of intrinsic variabilities 429 in different microstructure realizations of the same paste can be quantified. 430 The plot shows that finer dispersions of monosulfate have relatively more 431 accessible porosity near monosulfate surfaces. In particular, $\langle q \rangle$ increases 432 approximately linearly with the monosulfate fineness. For each unhydrated 433 microstructure, the slope of the relation was obtained by least-squares linear 434

regression. The mean of the three slopes is 0.189 with a relative standard uncertainty of about 10 %, measured as one standard deviation $(1-\sigma)$. Thefore, the relation between monosulfate fineness and water accessibility near the surface is unlikely to be an accidental result of one particular microstructural realization.

[Figure 8 about here.]

440

Recall that a voxel is considered damaged if the local tensile stress exceeds 441 the maximum tensile stress assumed for the phase occupying that voxel. The 442 severity of sulfate attack is now characterized by the number per unit volume 443 of damaged voxels in the microstructure, a quantity to which we hereafter 444 refer as simply "damage". Fig. 9 plots the damage as a function of $\langle g \rangle$ at dif-445 ferent ages of sulfate attack to investigate how monosulfate dispersion at the 44F beginning of sulfate attack influences the damage thereafter. The plots show 447 that, for any simulated exposure duration, expansion damage decreases with 448 increasing $\langle q \rangle$. In other words, damage is lessened by an increased availability 449 of capillary porosity near the monosulfate domains. In turn, this is accom-450 plished in part by having a finer dispersion of monosulfate throughout the 451 microstructure as opposed to fewer and larger occlusions. Fig. 10 confirms 452 that the predicted damage decreases approximately linearly with the initial 453 monosulfate domain number density, N_V . Again, separate linear regressions 454 were obtained for each microstructure realization at each exposure time, and 455 the relative standard uncertainty in the slope is always less than 10 %. Note 456 that monosulfate is completely consumed in these simulations by the end 457 of Stage 1, which is the first nine flushing cycles. Fig. 9 indicates that the 458

monosulfate distribution effect propagates for a significant time even after it 459 has disappeared from the system. This imprinted "memory" of the monosul-460 fate distribution is linked to the nature of the remaining capillary porosity 461 after monosulfate has been consumed. For example, a finer monosulfate dis-462 tribution, with relatively more accessible porosity nearby, enables ettringite 463 to fill that porosity with lower stress. Ettringite therefore fills the adjacent 464 capillary porosity more fully around finer monosulfate domains, leaving a 465 smaller capillary pore volume after monosulfate is consumed. This smaller 466 pore volume in turn implies (a) greater resistance to subsequent diffusion of 467 sulfate solution and (b) smaller amounts of sodium sulfate per unit volume 468 of microstructure that can cause further ettringite growth. 469

470 [Figure 9 about here.]

471 [Figure 10 about here.]

Such purely microstructural effects on sulfate attack, which are here pre-472 dicted to be responsible for up to a 26 % variation in damage, are never-473 theless of secondary importance compared to the influence of bulk cement 474 composition, as we investigate in the following section. However, these mi-475 crostructural influences have not received much attention previously and, 476 to our knowledge, have not been tested experimentally. These simulations 477 should therefore provide motivation for experimental tests of monsulfate dis-478 tribution and its effects on sulfate attack progress. Such experiments are 479 likely to be challenging because they require (a) the ability to clearly dif-480 ferentiate monosulfate from other AFm and AFt phases, (b) the ability to 481 map out the 3D distribution of monosulfate in a microstructure, and (c) the 482

ability to manipulate, to some extent, the monosulfate distribution as an
experimental variable. In principle, such experiments might be undertaken
using a combination of scanning electron microscopy (SEM) in backscattered
mode, combined with energy dispersive X-ray spectroscopy to identify elemental concentrations. Stereological principles could be used to statistically
reconstruct 3D distributions from 2D SEM/EDS images.

489 3.4. Influence of cement composition

Resistance to sulfate attack is directly linked to the aluminate content of 490 the cement. The CCRL 168 cement already discussed has a total equivalent 491 $\rm Al_2O_3$ content of 2.1 % by mass. In this section, we investigate sulfate attack 492 in two other cements: (1) a second Type I portland cement from the CCRL 493 proficiency program, designated here as CCRL 140, with a total equivalent 494 Al_2O_3 content of 1.7 %, and (2) a white cement ($Al_2O_3 = 0.5$ %). The mineral 495 composition and total aluminum content of all three cements are shown in 496 Table 1. 497

Figures 3 (for CCRL 168) and 11 (for the other two cements) show that 498 these three cements behave differently both during hydration and during ex-499 posure to sulfate solution. Gypsum is completely consumed from CCRL 140 500 and CCRL 168 within the first day of hydration, while a small amount of 501 gypsum remains in the white cement through 100 d of hydration. This rem-502 nant gypsum prevents the conversion of ettringite to monosulfate in the white 503 cement, in contrast to both CCRL 140 and CCRL 168. At the beginning of 504 sulfate attack, monosulfate and hydrogarnet (C_3AH_6) are the only aluminate-505 bearing phases predicted in CCRL 140. In CCRL 168, as already discussed, 506 monosulfate, hemicarbonate, monocarbonate, and hydrotalcite all contribute 507

to growth of ettringite during sulfate attack. As expected, the white cement, 508 with no aluminate-bearing phases other than ettringite at the end of hydra-509 tion, is essentially immune to sulfate attack. CCRL 168 has higher total Al 510 content than CCRL 140, and therefore a greater volume of ettringite formed 511 at the end (compare to Fig. 3(a)). This should be expected on thermody-512 namic grounds because the form of intermediate aluminate-bearing phases, 513 unstable with respect to ettringite, should not influence the ultimate amount 514 of ettringite formed. An interesting feature of CCRL 140 is that, with the 515 first introduction of external sulfate, monosulfate actually grows temporarily 516 at the expense of hydrogarnet. After hydrogarnet is consumed, monosulfate 517 transforms continuously to ettringite as expected. Overall, more rapid et-518 tringite growth is observed in CCRL 140 than in CCRL 168 during the early 519 stages of sulfate attack. Ettringite growth in CCRL 168 is initially slower 520 because its aluminates are partially contained in langbeinite, and because it 521 contains small amounts of calcite and periclase, both of which promote the 522 formation of carbonate and hydrotalcite phases that resist conversion to et-523 tringite more strongly than does monosulfate (compare Fig. 3 and Fig. 11(c)). 524 Therefore, in agreement with recent studies of sulfate attack [3, 15], these sim-525 ulations indicate that both limestone (either calcium carbonate or dolomitic) 526 and soluble magnesium sources can impart additional resistance to sulfate 527 attack. 528

The changes in ettringite mass during sulfate attack simulations are compared for all three cement pastes in Fig. 12. This figure shows the delay in ettringite conversion exhibited by CCRL 140 owing to the initial conversion of hydrogarnet to monosulfate. However, after complete conversion of

hydrogarnet, both the rate of conversion to ettringite and the final mass frac-533 tion of ettringite are greater in CCRL 140 than in CCRL 168. This result 534 might seem unusual at first sight based on the greater combined content of 535 iron and aluminum in CCRL 168. But as noted already in Fig. 3, a significant 536 portion of the iron in CCRL 168 is tied up in an iron-rich hydrotalcite, and 537 this iron stabilizes the hydrotalcite phase relative to ettringite. The paste 538 made from CCRL 140, in contrast, lacks a sufficient amount of magnesium 539 to form much hydrotalcite, so ettringite is the only aluminum/iron-bearing 540 phase remaining after prolonged exposure to sulfate solution. 541

This influence of cement chemistry on the predicted amounts of ettrin-542 gite is directly reflected in the predicted expansion and damage for all three 543 cement pastes, as shown in Fig. 13. As described earlier, the white cement 544 undergoes relatively little expansion because the aluminates are already com-545 pletely converted to ettringite by the time sulfate exposure has begun. Ex-546 pansion in the white cement is therefore due entirely to the relatively low 547 volume of gypsum precipitation. CCRL 168, with the highest aluminate 548 content of the three cements, sustains the greatest strain and damge over the 540 first 40 flushing cycles, However, expansion and damage in CCRL 140 reach 550 and exceed that in CCRL 168 after that point, due to its greater quantity of 551 monosulfate (15.9 % by mass) compared to CCRL 168 (7.2 % by mass). 552

In summary, the combined aluminum and iron content of a portland cement is an important, but not the only, factor in determining a paste's resistance to external attack by sodium sulfate solution. The presence of magnesium and carbonate components in the cement can promote formation of intermediate aluminate-bearing phases, such as hemicarbonate and hydrotalcite, that have a lower thermodynamic driving force for transforming to ettringite than phases such as monosulfate and other AFm phases.

560	[Figure 11 about here.]
561	[Figure 12 about here.]
562	[Figure 13 about here.]

563 4. Conclusion

The method used here to model sulfate attack is well suited for revealing 564 both chemical and microstructural influences on sulfate attack resistance for 565 at least three reasons. First, the model uses thermodynamic geochemical 566 speciation calculations to predict the equilibrium volumes and compositions 567 of hydrated phases during hydration and during subsequent intrusion by a 568 sodium sulfate solution. Second, it captures the 3D distribution of these 560 hydrates in a manner that has been shown to faithfully reproduce a range of 570 microstructural and macroscopic properties of the material [39, 45]. Third, 571 the model is directly coupled to a 3D finite element model that enables the 572 evolving stress field to be tracked as phase transformations give rise to local 573 misfit strain. Regions susceptible to damage can be identified and followed 574 based on a phase-dependent maximum tensile stress criterion. This latter 575 feature enables the model to compute the evolving effective elastic stiffness 576 of the binder and therefore monitor the material's resistance to expansion as 577 well as the driving forces for that expansion. It is worth emphasizing that 578 this is the first model of which we are aware that can provide a detailed 579

microstructural analysis of sulfate attack at the scale of individual cement
 grains.

The model reproduces primary effects of bulk chemistry on sulfate attack resistance that have been reported previously [15]. Specifically, ettringite growth in confined pore volumes is largely responsible for expansion and damage formation. Any changes in bulk chemistry that favor carboaluminate and hydrotalcite relative to monosulfate can reduce the material's susceptibility to sulfate attack compared to those that are rich in monosulfate.

The simulations also predict secondary microstructural influences on sul-588 fate attack resistance. Finer dispersions of monosulfate are predicted to 589 produce lower expansion and damage due to the greater relative accessi-590 bility of capillary porosity near the transforming monosulfate surfaces that 591 can accommodate excess ettringite volume. These microstructural details 592 have not received much attention previously and, to our knowledge, have 593 not been tested experimentally. But the model suggests that they can in-594 fluence macrosopic expansion by as much as 16% in the systems examined 595 here. New opportunities could be created for increasing sulfate resistance 596 in concrete if these aspects of hydrated microstructure could be controlled 597 more accurately, perhaps with organic additives that control nucleation and 598 growth of monosulfate. 599

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604 References

- [1] M. Santhanam, M. D. Cohen, J. Olek, Mechanism of sulfate attack: A
 fresh look Part 2. proposed mechanisms, Cem. Concr. Res. 33 (2003)
 341–346.
- [2] M. Santhanam, M. D. Cohen, J. Olek, Effects of gypsum formation on
 the performance of cement mortars during external sulfate attack, Cem.
 Concr. Res. 33 (2003) 325–332.
- [3] E. F. Irassar, V. L. Bonavetti, M. González, Microstructural study of
 sulfate attack on ordinary and limestone portland cements at ambient
 temperature, Cem. Concr. Res. 33 (2003) 31–41.
- [4] R. Tixier, B. Mobasher, Modeling of damage in cement-based materials
 subjected to external sulfate attack. I: Formulation, J. Mater. Civ. Eng.
 15 (4) (2003) 305–313.
- [5] R. Tixier, B. Mobasher, Modeling of damage in cement-based materials
 subjected to external sulfate attack. II: Comparison with experiments,
 J. Mater. Civ. Eng. 15 (4) (2003) 314–322.
- [6] P. J. M. Monteiro, K. E. Kurtis, Time to failure for concrete exposed to
 severe sulfate attack, Cem. Concr. Res. 33 (2003) 987–993.
- [7] P. Chindaprasirt, S. Homwuttiwong, V. Sirivivatnanon, Influence of
 fly ash fineness on strength, drying shrinkage and sulfate resistance of
 blended cement mortar, Cem. Concr. Res. 34 (2004) 1087–1092.

- [8] A. Neville, The confused world of sulfate attack on concrete, Cem.
 Concr. Res. 34 (2004) 1275–1296.
- [9] B. Mobasher, C. F. Ferraris, Simulation of expansion in cement based
 materials subjected to external sulfate attack, in: Proceedings of RILEM
 Technical Meeting on Durability of Cement Based Materials, 2004.
- [10] P. N. Gospodinov, Numerical simulation of 3D sulfate ion diffusion and
 liquid push out of the materials capillaries in cement composites, Cem.
 Concr. Res. 35 (2005) 520–526.
- [11] N. M. Al-Akhras, Durability of metakaolin concrete to sulfate attack,
 Cem. Concr. Res. 36 (2006) 1727–1734.
- [12] M. A. Shazali, M. H. Baluch, A. H. Al-Gadhib, Predicting residual
 strength in unsaturated concrete exposed to sulfate attack, J. Mater.
 Civ. Eng. 18 (2006) 343–354.
- [13] P. J. M. Monteiro, Scaling and saturation laws for the expansion of
 concrete exposed to sulfate attack, Proc. Nat. Acad. Sci. USA 103 (31)
 (2006) 11467–11472.
- [14] S. Sulfate resistance alkali silica Anantharaman, and resis-641 tance of class C & F replaced blended cements, M.Sc. the-642 Arizona State University, Phoenix, Arizona (May sis.2008). 643 Available http://faculty.engineering.asu.edu/cement/wpat 644 content/uploads/2011/09/Thesis_Sudheen_Print_021508.pdf (last 645 checked July 23, 2015). 646

- [15] T. Schmidt, B. Lothenbach, M. Romer, J. Neuenschwander, K. L.
 Scrivener, Physical and microstructural aspects of sulfate attack on ordinary and limestone blended Portland cements, Cem. Concr. Res. 39
 (2009) 1111–1121.
- [16] S. L. Sarkar, S. Mahadevan, J. C. L. Meeussen, H. van der Sloot, D. S.
 Kosson, Numerical simulation of cementitious materials degradation under external sulfate attack, Cem. Concr. Composites 32 (2010) 241–252.
- [17] A. E. Idiart, C. M. López, I. Carol, Chemo-mechanical analysis of concrete cracking and degradation due to external sulfate attack: A mesoscale model, Cem. Concr. Composites 33 (2011) 411–423.
- [18] W. Kunther, B. Lothenbach, K. L. Scrivener, Influence of bicarbonate
 ions on the deterioration of mortar bars in sulfate solutions, Cem. Concr.
 Res. 44 (2013) 77–86.
- [19] W. Kunther, B. Lothenbach, K. L. Scrivener, On the relevance of volume
 increase for the length changes of mortar bars in sulfate solutions, Cem.
 Concr. Res. 46 (2013) 23–29.
- [20] W. Kunther, B. Lothenbach, K. L. Scrivener, Deterioration of mortar
 bars immersed in magnesium containing sulfate solutions, Mater. Structures 46 (2013) 2003–2011.
- [21] C. Yu, W. Sun, K. L. Scrivener, Mechanism of expansion of mortars
 immersed in sodium sulfate solutions, Cem. Concr. Res. 43 (2013) 105–
 111.

- [22] I. Oliveira, S. H. P. Cavalaro, A. Aguado, New kinetic model to quantify
 internal sulfate attack in concrete, Cem. Concr. Res. 43 (2013) 95–104.
- [23] P. W. Brown, H. F. W. Taylor, The role of ettringite in external sulfate attack, in: J. Marchand, J. P. Skalny (Eds.), Materials Science
 of Concrete: Sulfate Attack Mechanisms, American Ceramic Society,
 Westerville, OH, 1999, pp. 73–98.
- [24] J. R. Clifton, J. M. Pommersheim, Sulfate attack of cementi-675 NIST tious materials: Volumetric relations and expansions, 676 Interagency Report 5390.National Institute of Standards 677 and Technology, Gaithersburg, MD (April 1994). Available at 678 http://fire.nist.gov/bfrlpubs/build94/PDF/b94040.pdf (last checked 679 July 23, 2015). 680
- [25] R. S. Gollop, H. F. W. Taylor, Microstructural and microanalytical studies of sulfate attack I. ordinary portland cement paste, Cem. Concr. Res.
 22 (1992) 1027–1038.
- [26] B. Tian, M. D. Cohen, Does gypsum formation during sulfate attack on
 concrete lead to expansion?, Cem. Concr. Res. 30 (2000) 117–123.
- [27] H. Lafuma, Théorie de l'expansion des ligants hydrauliques, Rev. Mater.
 Constr. Trav. Publics 243 (1929) 441–444.
- [28] M. D. Cohen, C. W. Richards, Effects of the particle sizes of expansive clinker on strength–expansion characteristics of type K expansive
 cements, Cem. Concr. Res. 12 (1982) 717–725.

- [29] P. K. Mehta, Chemistry and microstructure of expanseive cements, in:
 Proceedings of the Conference on Expansive Cement Concretes, University of California, Berkeley, 1972.
- [30] P. K. Mehta, Mechanism of expansion associated with ettringite formation, Cem. Concr. Res. 3 (1973) 1–6.
- [31] G. W. Scherer, Stress from crystallization of salt, Cem. Concr. Res. 34
 (2004) 1613–1624.
- [32] G. W. Scherer, Crystallization in pores, Cem. Concr. Res. 29 (1999)
 1347–1358.
- [33] R. J. Flatt, G. W. Scherer, Thermodynamics of crystallization stresses
 in DEF, Cem. Concr. Res. 38 (2008) 325–336.
- [34] E. Samson, J. Marchand, J. J. Beaudoin, Modeling the influence of
 chemical reactions on the mechanisms of ionic transport in porous materials: An overview, Cem. Concr. Res. 30 (2000) 1895–1902.
- [35] J. Marchand, Modeling the behavior of unsaturated cement systems exposed to aggressive chemical environments, Mater. Structures 34 (2001)
 195–200.
- [36] E. Samson, J. Marchand, Modeling the transport of ions in unsaturated
 cement-based materials, Computers & Structures 85 (2007) 1740–1756.
- [37] P. N. Gospodinov, R. F. Kazandjiev, T. A. Partalin, M. K. Mironova,
 Diffusion of sulfate ions into cement stone regarding simultaneous chem-

- ical reactions and resulting effects, Cem. Concr. Res. 29 (1999) 1591–
 1596.
- [38] F. Schmidt-Döhl, F. S. Rostásy, A model for the calculation of combined
 chemical reactions and transport processes and its application to the corrosion of mineral-building materials. Part II. Experimental verification,
 Cem. Concr. Res. 29 (1999) 1047–1053.
- [39] J. W. Bullard, B. Lothenbach, P. E. Stutzman, K. A. Snyder, Coupling
 thermodynamics and digital image models to simulate hydration and
 microstructure development of portland cement pastes, J. Mater. Res.
 26 (4) (2011) 609–622.
- [40] E. J. Garboczi, Finite element and finite difference programs for
 computing the linear electric and elastic properties of digital images
 of random materials, NISTIR 6269, U.S. Department of Commerce
 (Dec. 1998). Available at http://www.nist.gov/manuscript-publicationsearch.cfm?pub_id=860168 (last checked July 23, 2015).
- [41] L. J. Parrot, D. C. Killoh, Prediction of cement hydration, Br. Ceram.
 Proc. 35 (1984) 41–53.
- [42] D. A. Kulik, GEMS-PSI 2.03, http://les.web.psi.ch/Software/GEMSPSI/ (last checked July 23, 2015).
- [43] B. Lothenbach, F. Winnefeld, Thermodynamic modelling of the hydration of Portland cement, Cem. Concr. Res. 36 (2006) 209–226.

- [44] B. Lothenbach, T. Matschei, G. Möschner, F. P. Glasser, Thermodynamic modelling of the effect of temperature on the hydration and porosity of Portland cement, Cem. Concr. Res. 38 (2008) 1–18.
- [45] P. Feng, C. Miao, J. W. Bullard, A model of phase stability, microsructure and properties during leaching of portland cement binders, Cem.
 Concr. Composites 49 (2014) 9–19.
- [46] T. Sugiyami, R. Worapatt, T. Yukikazu, Simultaneous transport of chloride and calcium ions in hydrated cement systems, J. Adv. Concrete
 Technol. 1 (2003) 127–138.
- [47] J. G. Wang, Sulfate attack on hardened cement paste, Cem. Concr. Res.
 24 (4) (1994) 735–742.
- [48] E. Roziere, A. Loukili, R. E. Hachem, F. Grondin, Durability of concrete
 exposed to leaching and external sulphate attack, Cem. Concr. Res. 39
 (2009) 1188–1198.
- [49] W. Hummel, U. Berner, E. Curti, F. J. Pearson, T. Thoenen, Nagra /
 PSI Chemical Thermodynamic Data Base 01/01, Universal Publishers,
 Parkland, Florida, 2002.
- [50] C. J. Haecker, E. J. Garboczi, J. W. Bullard, R. B. Bohn, Z. Sun, S. P.
 Shah, T. Voigt, Modeling the linear elastic properties of Portland cement
 paste, Cem. Concr. Res. 35 (2005) 1948–1960.
- ⁷⁵³ [51] M. F. Thorpe, E. J. Garboczi, Elastic properties of central-force net⁷⁵⁴ works with bond-length mismatch, Phys. Rev. B 42 (1990) 8405–8417.

- ⁷⁵⁵ [52] A. P. Roberts, E. J. Garboczi, Elastic properties of a tungsten-silver
 ⁷⁵⁶ composite by reconstruction and computation, J. Mech. Phys. Solids 47
 ⁷⁵⁷ (1999) 2029–2055.
- [53] S. Meille, E. J. Garboczi, Linear elastic properties of 2D and 3D models
 of porous materials made from elongated objects, Modell. Simul. Mater.
 Sci. Eng. 9 (5) (2001) 371–390.
- ⁷⁶¹ [54] C. H. Arns, M. A. Knackstedt, W. V. Pinczewski, E. J. Garboczi,
 ⁷⁶² Computation of linear elastic properties from microtomographic images:
 ⁷⁶³ Methodology and agreement between theory and experiment, Geophys.
 ⁷⁶⁴ 67 (5) (2002) 1396–1405.
- [55] P. Sahachaiyunta, K. Ponpaisanseree, J. W. Bullard, P. E. Stutzman,
 E. J. Garboczi, W. Vichit-Vadakan, Virtual testing in a cement plant,
 Concr. Intl. (2012) 33–39.
- [56] L. Valentini, M. Parisatto, V. Russo, G. Ferrari, J. W. Bullard, R. J.
 Angel, M. C. Dalconi, G. Artioli, Prediction of the mechanical properties of industrial cement mortars by computer modelling, Cem. Concr.
 Composites in press.
- ⁷⁷² [57] J. M. Raphael, Tensile strength of concrete, ACI J. 81 (1984) 158–165.
- ⁷⁷³ [58] A. Nazari, S. Riahi, Splitting tensile strength of concrete using ground
 ⁷⁷⁴ granulated blast furnace slag and SiO₂ nanoparticles as binder, Energy
 ⁷⁷⁵ and Buildings 43 (2011) 864–872.
- ⁷⁷⁶ [59] T. Belytschko, R. Gracie, G. Bentura, A review of extended/generalized

- finite element methods for material modeling, Modelling Simul. Mater.
 Sci. Eng. 17 (2009) 043001.
- ⁷⁷⁹ [60] Annual Book of ASTM Standards, ASTM International, West Con-⁷⁸⁰ shohocken, PA, 2000.
- [61] AASHTO Standard T 353-14, Particle Size Analyis of Hydraulic Cement
 and Related Materials by Light Scattering, American Association of
 State Highway and Transportation Officials, Washington, D.C. (2014).
- ⁷⁸⁴ [62] Virtual cement and concrete testing laboratory software,
 http://www.nist.gov/el/buildint_materials/evcctl.cfm (June 2014).
- [63] C. F. Ferraris, P. E. Stutzman, M. A. Peltz, J. A. Winpigler, Developing
 a more rapid test to assess sulfate resistance of hydraulic cements, J. Res.
 National Inst. Standards Technol. 110 (5) (2005) 529–540.

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сѕн	СЅН	CSH		сѕн	сѕн	сѕн
сѕн	monosulphate	water	\Rightarrow	СЅН	ettringite	ettringite
сѕн	сѕн	сѕн		сѕн	сѕн	сѕн

Figure 1: Illustration of a local microstructure configuration where monosulfate could transform to ettringite.



Figure 2: Flowchart of sulfate attack model.



Figure 3: Predicted mass change with increased flushing cycles for solid phases (a) containing aluminum and (b) without aluminum.



Figure 4: Ionic concentrations in the capillary pore solution before and after equilibration with the solids, plotted against the number of flushing cycles.



Figure 5: Simulated evolution of pore solution pH over the course of prolonged exposure to $0.1\,\rm mol\,kg^{-1}$ sodium sulfate solution.





Figure 6: Evolution of CCRL 168 cement paste microstructure: (a) hydrated to 0.90 degree of hydration; (b) end of Stage 1, when monosulfatehas been converted to ettringite; (c) end of Stage 2, when all carboaluminates have been converted to ettringite; (d) magnified view of a portion of the microstructure in (c) showing the deformation around secondary ettringite. The displacement field is magnified by $5 \times$ to more clearly show deformation. Phase colors are $C_3S =$ brown, $C_2S =$ light blue, $C_3A =$ grey, $C_4AF =$ white, CSH = beige, CH = dark blue, calcite = green, ettringite = violet, monosulfate = olive, hydrotalcite = light green, capillary porosity = black.



Figure 7: Microstructural damage field (black) after (a) end of Stage 1 and (b) end of Stage 2.



Figure 8: Relation between number of monosulfate domains and $\langle g \rangle$ for water adjacent to monosulfate surfaces. Different symbols represent results from different microstructure realizations of the same starting cement paste. Least-squares linear regressions associated with each starting microstructure are shown as dashed lines. Standard uncertainty in the mean slope of 0.189 is reported in terms of one standard deviation.



Figure 9: Damage after different sulfate flushing cycles as a function of $\langle g \rangle$ for water near monosulfate domains. In each plot, different symbols are associated with different realizations of the same starting microstructure. Least-squares linear regression was used to obtain the slope (dashed lines) for each microstructure realization, and the mean slope and standard uncertainty $(1-\sigma)$ are displayed.



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Figure 11: Influence of Al content on phase evolution during hydration and subsequent external sulfate attack of a white cement (a,b) and CCRL 140 Type I cement (c,d).



Figure 12: Comparison of ettringite mass evolution during sulfate attack for cement pastes made from CCRL 168, CCRL 140, and the white cement.



Figure 13: Influence of Al content on average strain, $\langle \epsilon \rangle$ (top) and damage (bottom) during exposure to sodium sulfate solution.

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846		components [43] \ldots \ldots \ldots \ldots \ldots \ldots	53

Phase	CCRL 168	CCRL 140	White Cement		
C_3S	54.5	64.7	78.3		
β -C ₂ S	15.7	16.5	10.7		
C_3A	8.0	5.4	2.4		
C_4AF	7.0	7.7	0.9		
Bassanite	2.3	2.2	3.5		
Anhydrite	0.2	1.6	0.5		
Gypsum		0.4	0.6		
Arcanite	1.1	0.3	0.5		
Langbeinite	0.5				
Aphthitalite	1.3				
Periclase	3.2		0.2		
Calcite	1.1		1.3		
Total Al_2O_3	2.1	1.7	0.5		
Present as solid solution in clinker phases					
K_2O	0.325	0.320	0.217		
MgO	0.775	0.855	0.762		
SO_3	0.598	0.687	0.734		

Table 1: Mineralogical composition of the cements used in this study, expressed as mass percent.

Phase	Formula	$V_{\rm m}~(10^{-4}~{\rm m}^3)$
Ettringite	$Ca_6Al_2(SO_4)_3(OH)_{12} \cdot 26H_2O$	7.070
Fe-ettringite	$\operatorname{Ca}_{6}\operatorname{Fe}_{2}(\operatorname{SO}_{4})_{3}(\operatorname{OH})_{12} \cdot 26 \operatorname{H}_{2}\operatorname{O}$	7.176
Monosulfate	$\operatorname{Ca}_4\operatorname{Al}_2(\operatorname{SO}_4)(\operatorname{OH})_{12} \cdot 6\operatorname{H}_2\operatorname{O}$	3.090
Fe-monosulfate	$\operatorname{Ca}_{4}\operatorname{Fe}_{2}(\operatorname{SO}_{4})(\operatorname{OH})_{12} \cdot 6 \operatorname{H}_{2}\operatorname{O}$	3.211
Hemicarbonate	$Ca_4Al_2(CO_3)_{0.5}(OH)_{13} \cdot 5.5 H_2O$	2.845
Fe-hemicarbonate	$Ca_4Fe_2(CO_3)_{0.5}(OH)_{13} \cdot 5.5 H_2O$	2.965
Monocarbonate	$\operatorname{Ca}_4\operatorname{Al}_2(\operatorname{CO}_3)(\operatorname{OH})_{12} \cdot 5\operatorname{H}_2\operatorname{O}$	2.620
Fe-monocarbonate	$\operatorname{Ca}_{4}\operatorname{Fe}_{2}(\operatorname{CO}_{3})(\operatorname{OH})_{12} \cdot 5\operatorname{H}_{2}\operatorname{O}$	2.902
Hydrotalcite	$Mg_4Al_2(OH)_{14} \cdot 3H_2O$	2.202
Fe-hydrotalcite	$Mg_4Fe_2(OH)_{14} \cdot 3H_2O$	2.324
$\rm CO_3$ -hydrotalcite	$\mathrm{Mg}_4\mathrm{Al}_2(\mathrm{CO}_3)(\mathrm{OH})_{12}\cdot 2\mathrm{H}_2\mathrm{O}$	2.204

Table 2: Composition and molar volumes of main aluminate and iron components [43]